



**0,60kN pistekuorma 200mm, kisko**

**Calculation report №  
MQ-41\_3\_0.60kN p.kuorma etäisyys 200mm**

Performed by

20.12.2019

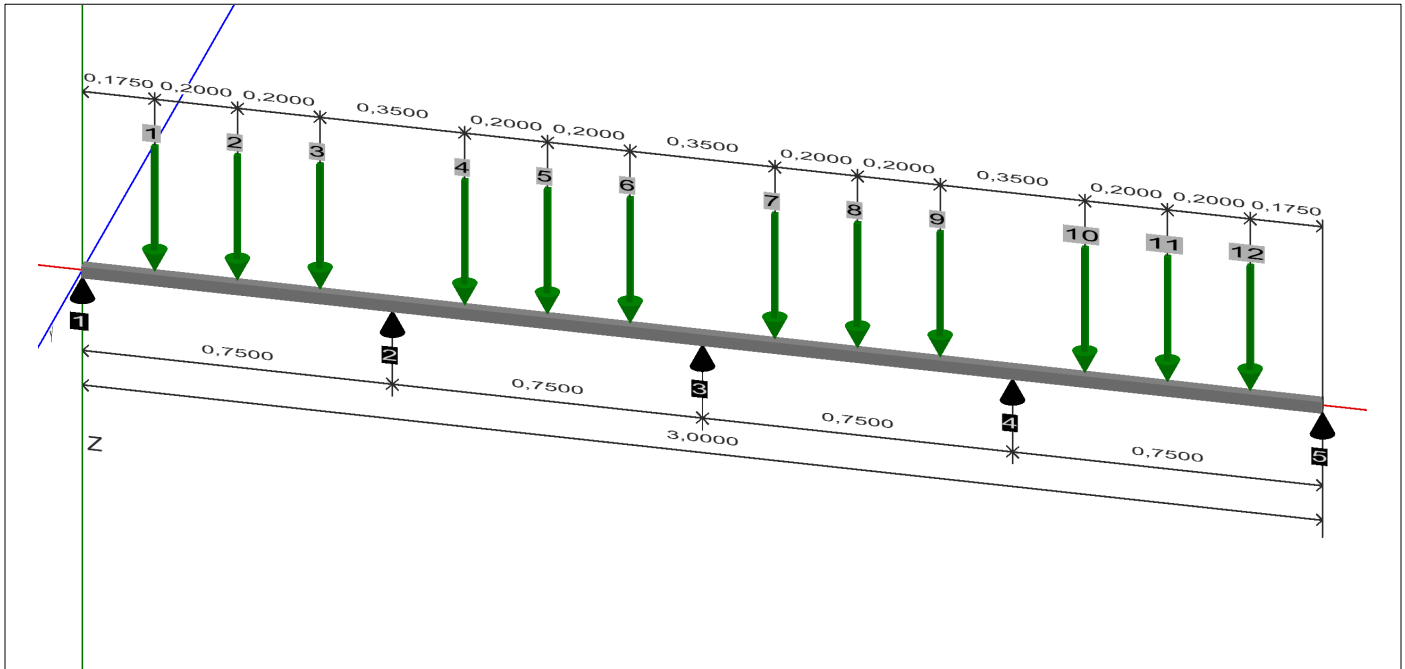
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## Project Project1

### Subproject 0,60kN pistekuorma 200mm, kisko

#### Statical system



Beam MQ-41/3 OK

#### Selected beam

| Channel | Length [m] | Rotation | A [mm <sup>2</sup> ] | I <sub>y</sub> [cm <sup>4</sup> ] | I <sub>z</sub> [cm <sup>4</sup> ] | E [N/mm <sup>2</sup> ] |
|---------|------------|----------|----------------------|-----------------------------------|-----------------------------------|------------------------|
| MQ-41/3 | 3,0000     |          | 375,88               | 7,70                              | 10,79                             | 210 000                |

A= Cross section area, I<sub>y</sub> I<sub>z</sub>= Moment of inertia, E= Modulus of elasticity

#### Supports

| Support No. |  | Distance from left A [m] |  | Span L [m] |  |
|-------------|--|--------------------------|--|------------|--|
| 1           |  | 0,0000                   |  | 0,7500     |  |
| 2           |  | 0,7500                   |  | 0,7500     |  |
| 3           |  | 1,5000                   |  | 0,7500     |  |
| 4           |  | 2,2500                   |  | 0,7500     |  |
| 5           |  | 3,0000                   |  | 0,0000     |  |

#### Loads

##### Single loads

| No. | Load type   | Position [m] | Forces [kN] |        |
|-----|-------------|--------------|-------------|--------|
|     |             |              | Y           | Z      |
| 1   | Design load | 0,1750       | 0,0000      | 0,6000 |
| 2   | Design load | 0,3750       | 0,0000      | 0,6000 |
| 3   | Design load | 0,5750       | 0,0000      | 0,6000 |

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### Single loads

| No. | Load type   | Position [m] | Forces [kN] |        |
|-----|-------------|--------------|-------------|--------|
|     |             |              | Y           | Z      |
| 4   | Design load | 0,9250       | 0,0000      | 0,6000 |
| 5   | Design load | 1,1250       | 0,0000      | 0,6000 |
| 6   | Design load | 1,3250       | 0,0000      | 0,6000 |
| 7   | Design load | 1,6750       | 0,0000      | 0,6000 |
| 8   | Design load | 1,8750       | 0,0000      | 0,6000 |
| 9   | Design load | 2,0750       | 0,0000      | 0,6000 |
| 10  | Design load | 2,4250       | 0,0000      | 0,6000 |
| 11  | Design load | 2,6250       | 0,0000      | 0,6000 |
| 12  | Design load | 2,8250       | 0,0000      | 0,6000 |

### Calculation summary

Beam MQ-41/3 OK

|                            |       |
|----------------------------|-------|
| Deflection utilization [%] | 7,75  |
| Stress utilization [%]     | 19,36 |

### Calculation factors

|                                |               |
|--------------------------------|---------------|
| Design basis:                  | Eurocode 1993 |
| Load combination design basis: | Eurocode 1990 |
| L1                             | Dead load     |
| L2                             | Live load     |
| L3                             | Design load   |

Load combinations:

ULS

$$LC1-ULS = 1,35 * L1 + 1,50 * L2$$

$$LC2-ULS = 1,35 * L1 + 1,00 * L3$$

SLS

$$LC1-SLS = 1,00 * L1 + 1,00 * L2$$

$$LC2-SLS = 0,90 * L1 + 0,67 * L3$$

|  |       |
|--|-------|
| Partial safety factors material $\gamma_M$ : | 1,1   |
| Maximum beam allowable deflection:           | L/200 |
| Maximum cantilever allowable deflection      | L/150 |
| Min. deflection limit [mm]                   | 1,5   |

### Detailed results

| Support position [m] | Length [m] | Force at. supp. point [kN] |         |        |         | Bending moment [kNm] |         |        |         |
|----------------------|------------|----------------------------|---------|--------|---------|----------------------|---------|--------|---------|
|                      |            | Z                          | LC      | Y      | LC      | My                   | LC      | Mz     | LC      |
| 0,0000               | 0,7500     | 0,6780                     | LC2-ULS | 0,0000 | LC2-ULS | 0,1770               | LC2-ULS | 0,0000 | LC1-ULS |

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| Support position [m] | Length [m] | Force at. supp. point [kN] |         |        |         | Bending moment [kNm] |         |        |         |
|----------------------|------------|----------------------------|---------|--------|---------|----------------------|---------|--------|---------|
|                      |            | Z                          | LC      | Y      | LC      | My                   | LC      | Mz     | LC      |
| 0,7500               | 0,7500     | 2,1430                     | LC2-ULS | 0,0000 | LC2-ULS | 0,1770               | LC2-ULS | 0,0000 | LC1-ULS |
| 1,5000               | 0,7500     | 1,6750                     | LC2-ULS | 0,0000 | LC2-ULS | 0,1770               | LC2-ULS | 0,0000 | LC1-ULS |
| 2,2500               | 0,7500     | 2,1430                     | LC2-ULS | 0,0000 | LC2-ULS | 0,1770               | LC2-ULS | 0,0000 | LC1-ULS |
| 3,0000               | 0,7500     | 0,6780                     | LC2-ULS | 0,0000 | LC2-ULS |                      |         |        |         |

| Support position [m] | Length [m] | Bending stress [N/mm <sup>2</sup> ] |
|----------------------|------------|-------------------------------------|
| 0,0000               | 0,7500     | 51                                  |
| 0,7500               | 0,7500     | 51                                  |
| 1,5000               | 0,7500     | 51                                  |
| 2,2500               | 0,7500     | 51                                  |
| 3,0000               | 0,7500     | 51                                  |

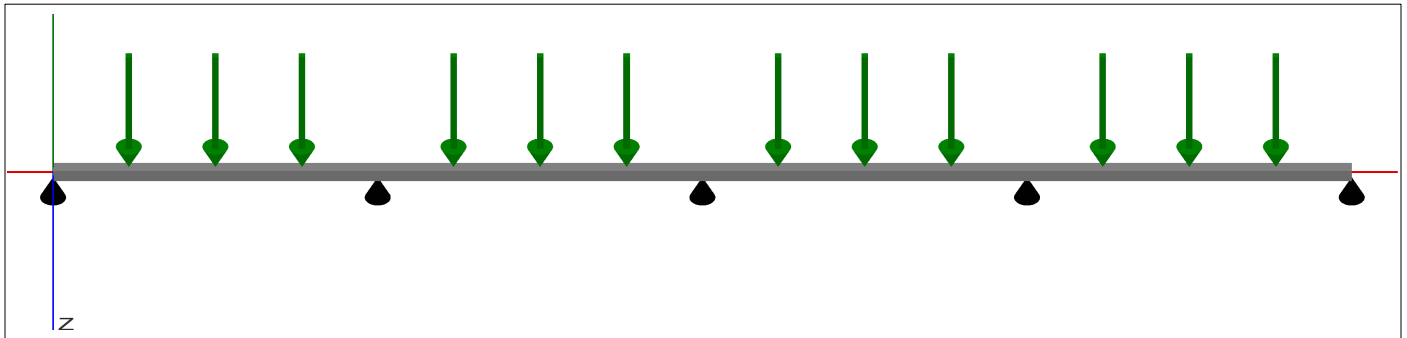
| Support position [m] | Length [m] | Deflection [mm] |         |     |         |
|----------------------|------------|-----------------|---------|-----|---------|
|                      |            | Z               | LC      | Y   | LC      |
| 0,0000               | 0,7500     | 0,3             | LC2-SLS | 0,0 | LC2-SLS |
| 0,7500               | 0,7500     | 0,1             | LC2-SLS | 0,0 | LC2-SLS |
| 1,5000               | 0,7500     | 0,1             | LC2-SLS | 0,0 | LC2-SLS |
| 2,2500               | 0,7500     | 0,3             | LC2-SLS | 0,0 | LC2-SLS |
| 3,0000               | 0,7500     |                 |         |     |         |

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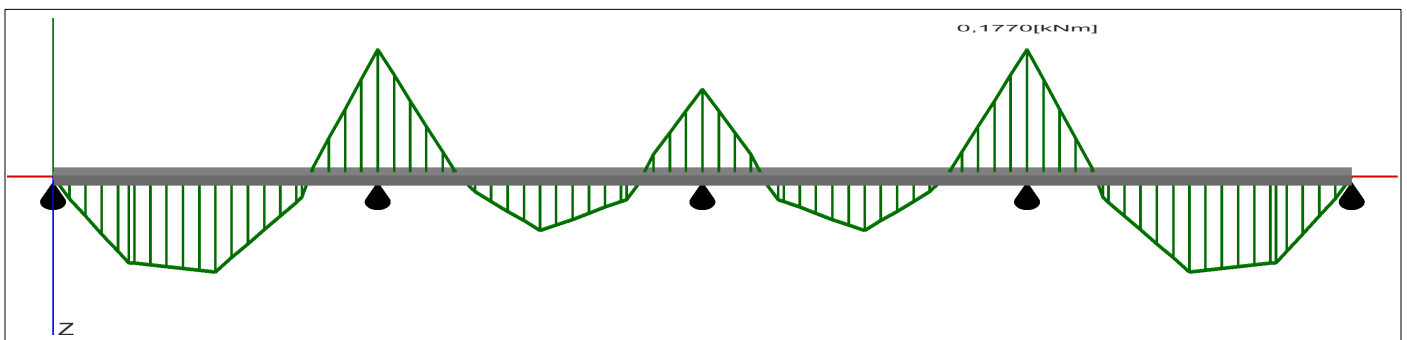
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## Diagrams / Charts

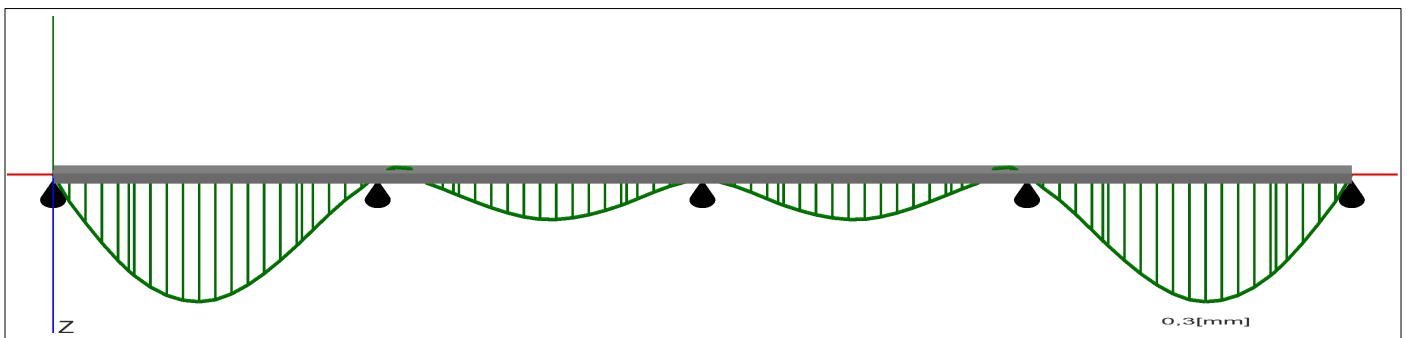
Planning view



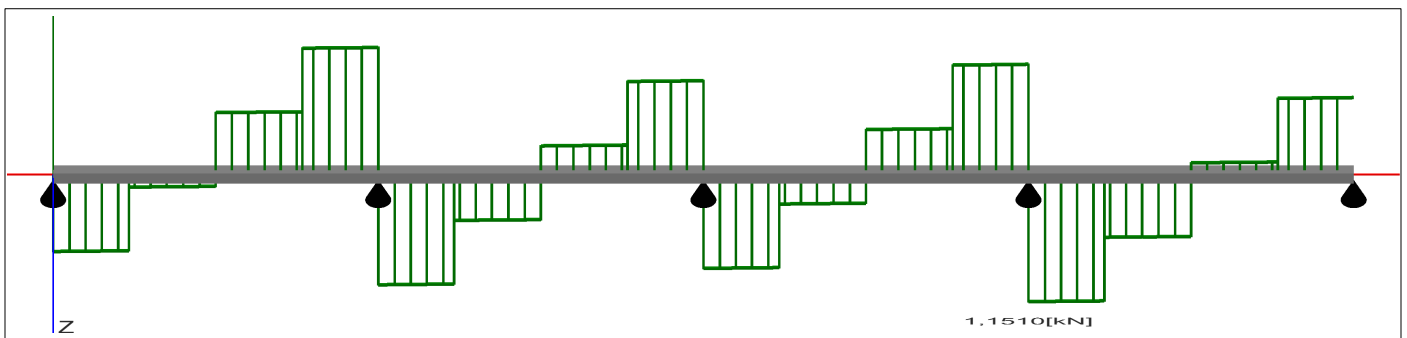
Bending moment (Load combination Z: LC2-ULS Y: LC1-ULS )



Deflection (Load combination Z: LC2-SLS Y: LC2-SLS )



Shear load (Load combination Z: LC2-ULS Y: LC1-ULS )



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### **General design note**

Channel design computation is carried out by the calculation engine from the RSTAB 8.04.0131.84645 framework software by Dlubal, analogous to the elastic-elastic method in conformance with EC3/DIN 18800 for Europe and AISI S100 for the US. The connector design method is based on a combination of several calculation models following:

- for Europe the principles of either DIN 18800 or EC 3 and tests carried out by an independent institute (HTL Rankweil, Austria).
- for US the principles of AISC 360 13th Edition and tests carried out by an independent institute (HTL Rankweil, Austria)

The static analysis is performed on the basis of a stationary system. 2nd-Order analysis due to possible eccentricities or deflections in the design (deformation according to DIN 18800 or EC3) must be considered separately by the appropriate personnel.

Only channel sections and standard cantilevers are verified. Connectors need to be checked separately.

Buckling and LTB checks must always be controlled separately by the responsible design engineer.

Local stress and deformation of members at supporting points and loading positions is not considered.

Relative deflection evaluation and stability checks: For the relative deflection evaluation and stability checks PROFIS Installation uses a reference length based on a set of members. A member is a connection from one node to the next on a beam. Members can be connected to a set of members if the nodes in between do not reduce the reference length. This connection of members to a set of members is done automatically based on the assumption that a node with very low global displacement is either a support or can be regarded as a support. The global displacement limit to define a node as a support is 0.1 mm for relative deflection evaluation and 0.005 for stability checks. The connection of members to a set of members can also be done by the user. The user can also decide manually if a set of members is a single-/multispan beam or a cantilever. The buckling ratio can also be manually changed. The user can finally also decide to exclude a set of members from the relative deflection evaluation. In case of any manual adjustment you will find a remark in the report.

The design must be checked for its plausibility before assembly.

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